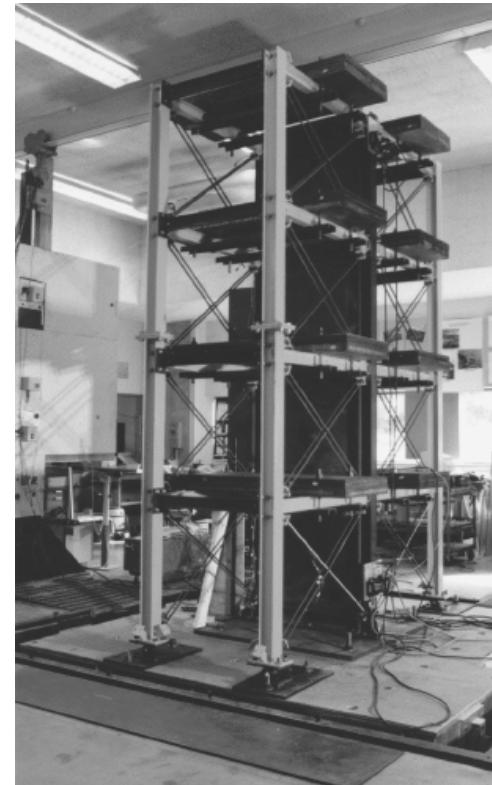
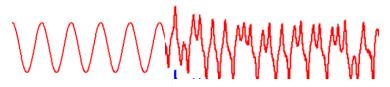


VIBRAÇÕES EM ESTRUTURAS DE AÇO

Zacarias M. Chamberlain Pravia (Ver. 2020)




Zacarias Chamberlain 2020



***ESTE MATERIAL É DE PROPRIEDADE DO AUTOR E APENAS e É FORNECIDO
COMO FERRAMENTA DE ACOMPANHAMENTO DO CONTEÚDO DO CURSO.***



J dfdukdv#kdp ehuolq#353

CONTEÚDO

1 - Introdução as vibrações, porque é necessário saber dinâmica?

2 - Sistemas de Um grau de liberdade

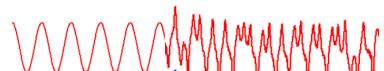
2.1 – Vibrações livres – equação de movimento

2.2 – Vibração livre não amortecida (frequência natural)

2.3 – Vibração livre amortecida

2.4 - Determinação de frequências naturais cálculo manual

2.5 - Determinação de frequências naturais usando programas computacionais



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CONTEÚDO

3 – Sistemas de múltiplos graus de liberdade

3.1 – Sistema não amortecido

3.2 - Frequências e modos de vibração

3.3 – Análise Modal de Vibrações livres

3.4 – Análise Modal de vibrações forçadas

CONTEÚDO

3 – Problemas dinâmicos nas estruturas de aço e soluções

3.1 – Vibrações induzidas por equipamentos rotativos

3.2 – Vibrações induzidas pelo vento

3.3 – Vibrações produzidas por movimento humano (ginásios, passarelas, pisos)

3.4 – Vibrações produzidas na construção civil

5.6 - Medições de frequências naturais.

4 – Medição de frequências Naturais (aula prática)

REFERÊNCIAS

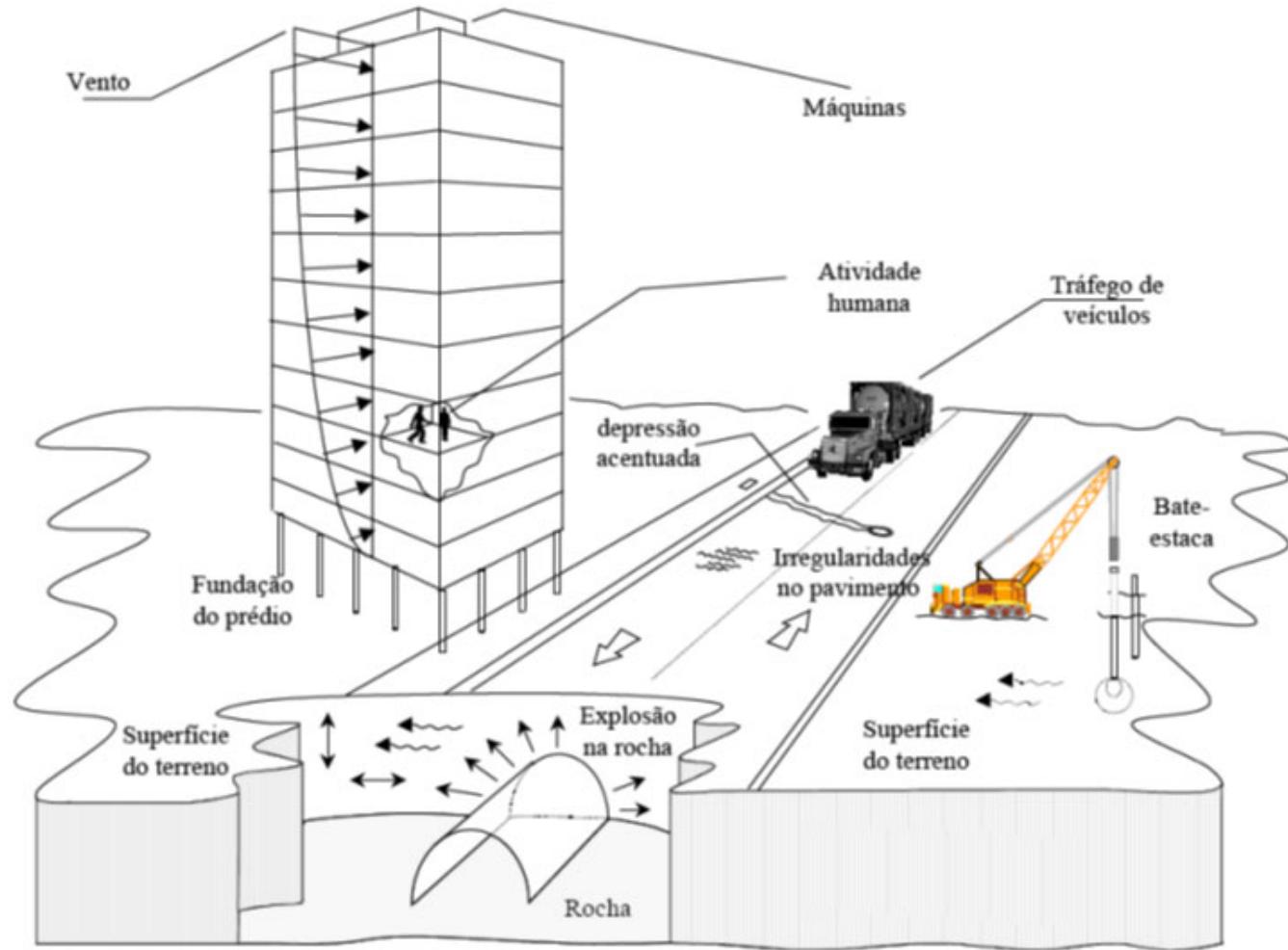
[1] BRASIL, R., SILVA, M.A., **Introdução à dinâmica das estruturas para Engenharia Civil**, 2^a edição, Editora Edgar Blucher, 2017.

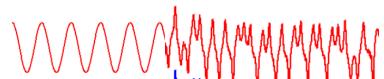
[2] SORIANO, H. LIMA, **Introdução à dinâmica das estruturas**, ELSEVIEAR Editora Limitada, 2014.

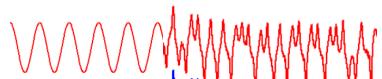
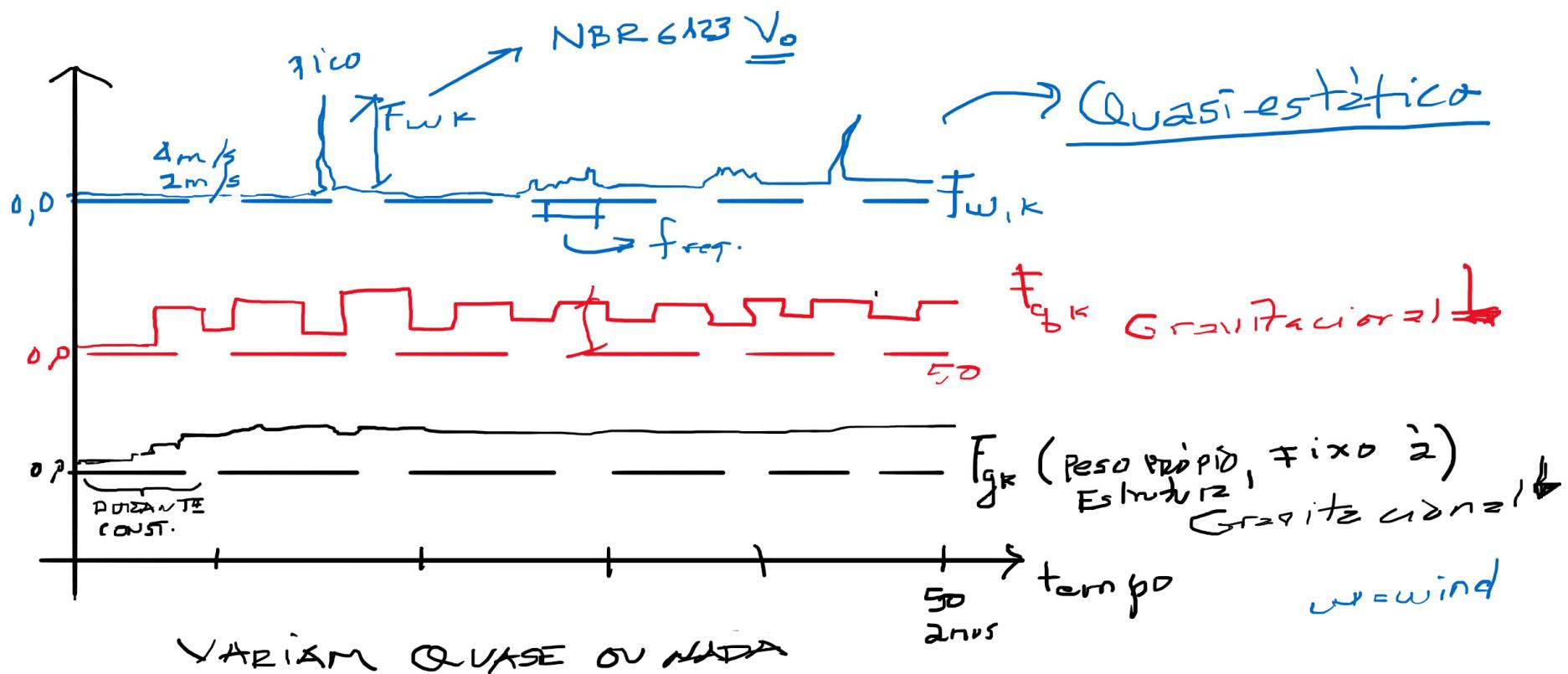
[3] Vibrações em Pavimentos: Recomendações Técnicas de Projecto, RFS2-CT-2007-00033

[[4] Murray et al, *Vibrations of Steel-Framed Structural Systems Due to Human Activity*, Second edition, AISC Design Guide, 2016.

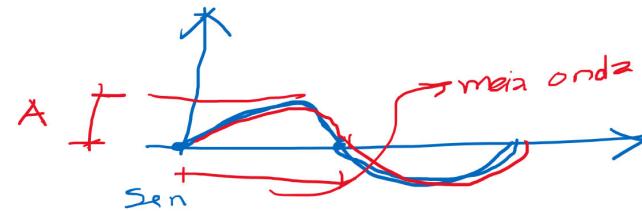
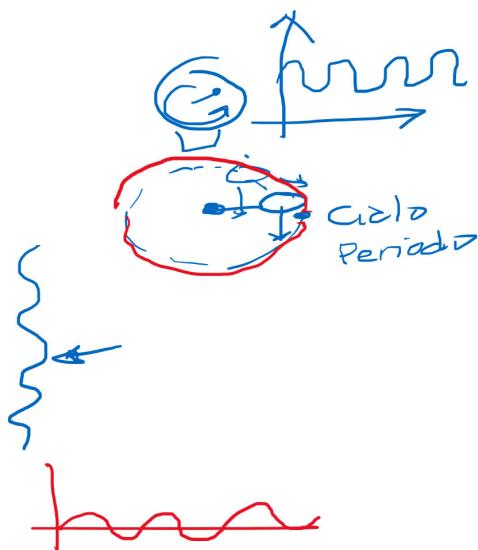
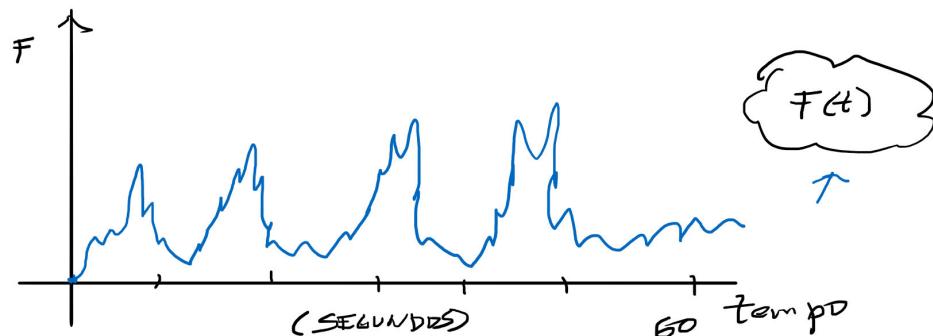
(1) PORQUÊ VIBRAÇÕES?




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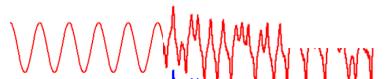
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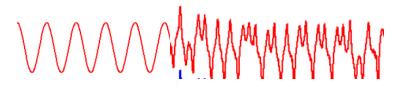
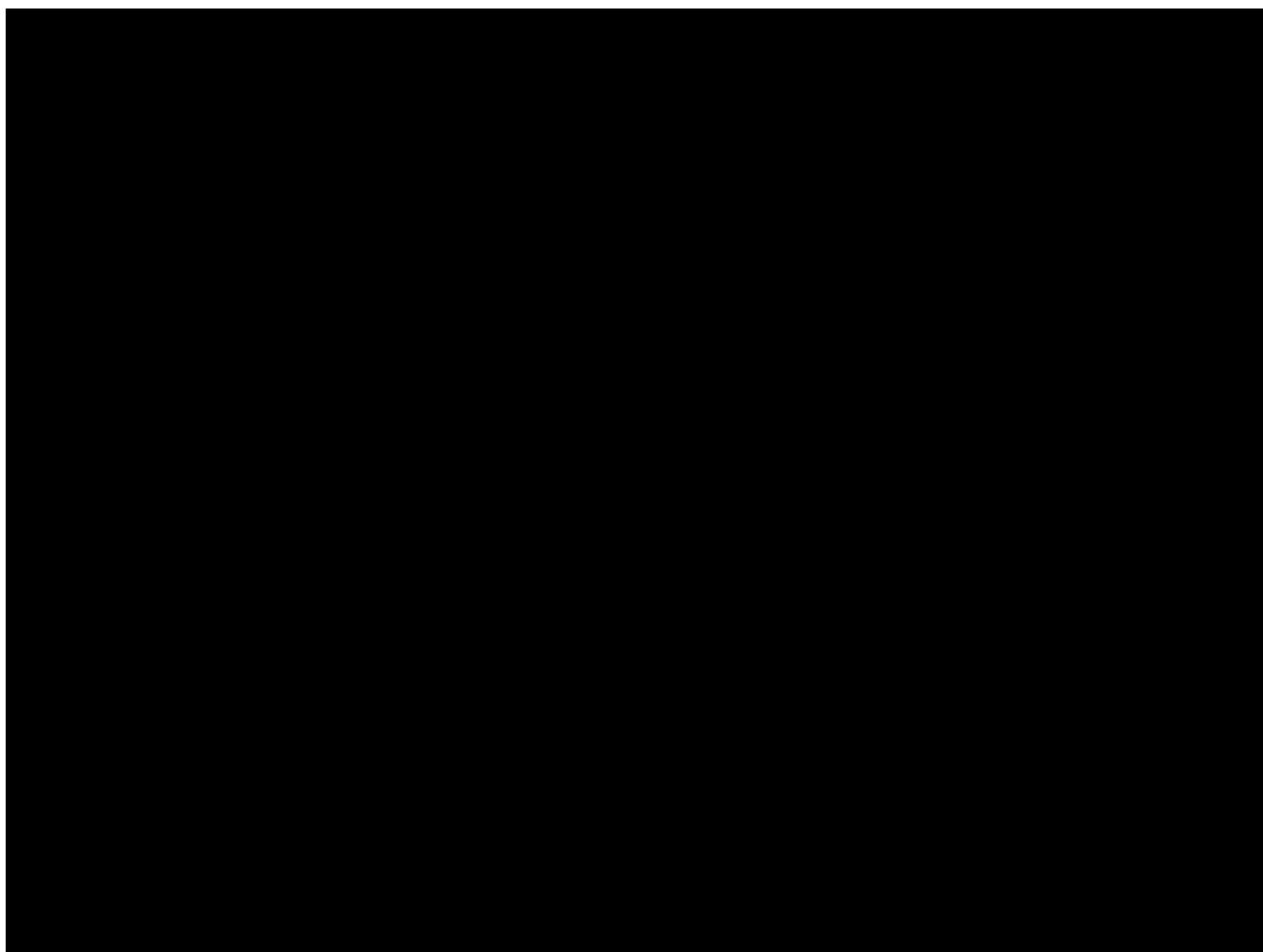


$$T = 60 \text{ seg}$$

$$f = \frac{1}{T} = \frac{1 \text{ ciclos}}{60 \text{ seg}} = \frac{1}{60}$$

$$\frac{1}{s} = \text{Her}^-$$


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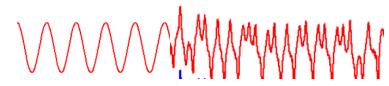


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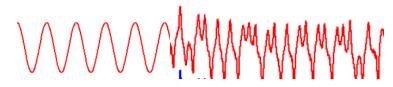
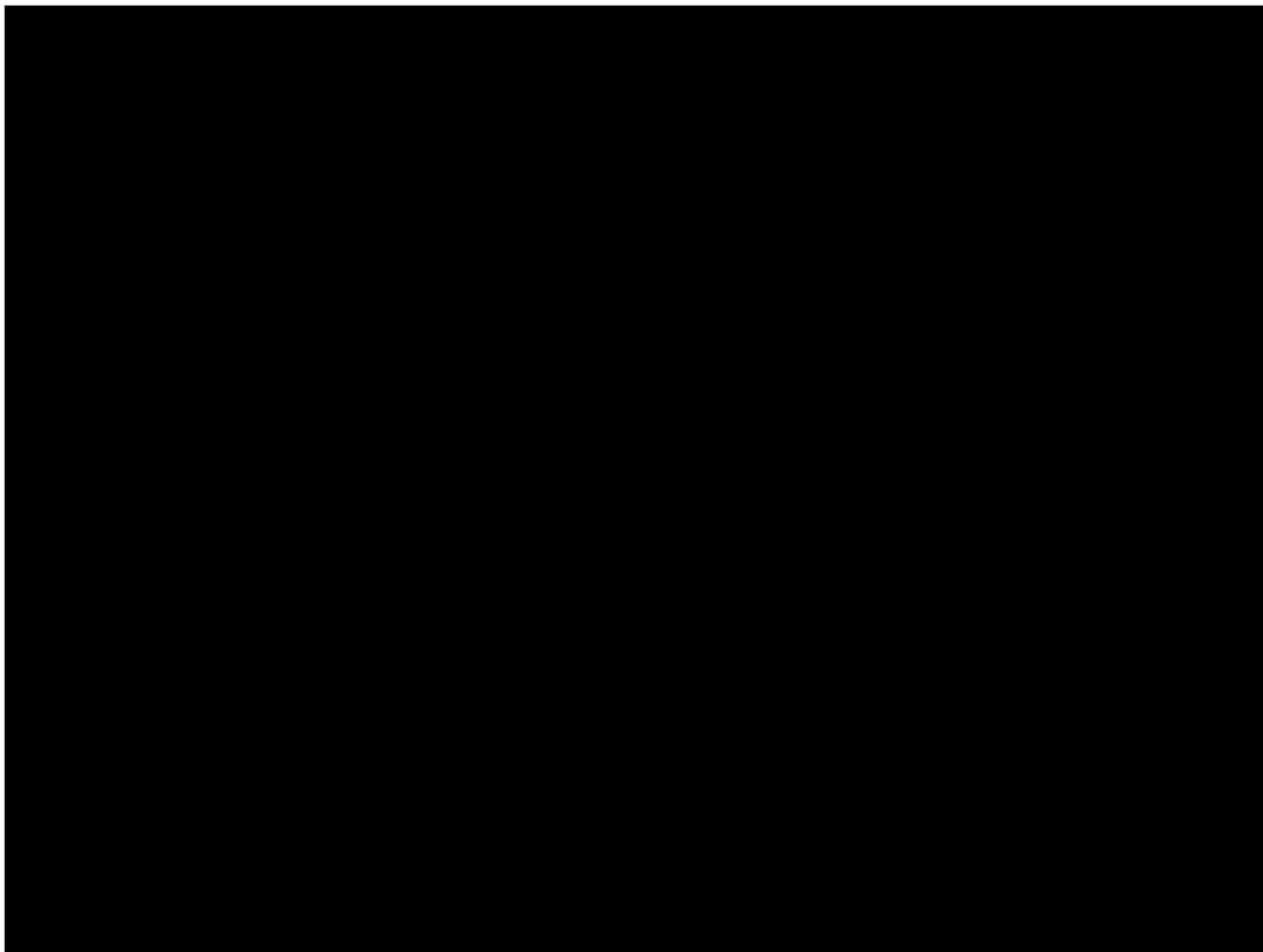
A red graphic element consisting of a series of wavy, horizontal lines of varying lengths, resembling a stylized waveform or a series of short marks. Below this graphic, the name "Zacarias Chamberlain" is written in a black, sans-serif font, followed by the year "2020" in a smaller size.

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JC - Pista 2

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16-10-97



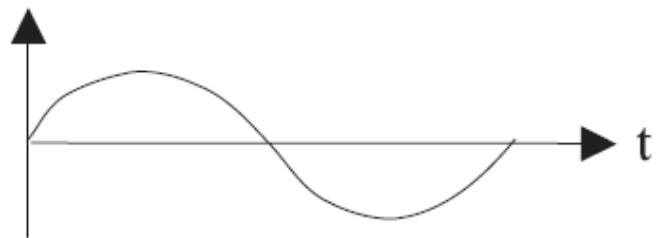
Zacarias Chamberlain 2020



Zacarias Chamberlain 2020

A red graphic element consisting of a series of wavy, horizontal lines of varying lengths, resembling a stylized waveform or a series of short marks. Below this graphic, the name "Zacarias Chamberlain" is written in a black, sans-serif font, followed by the year "2020" in a smaller size.

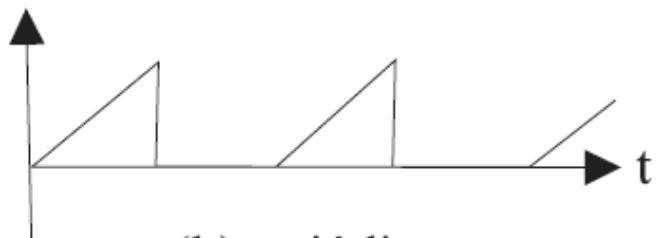




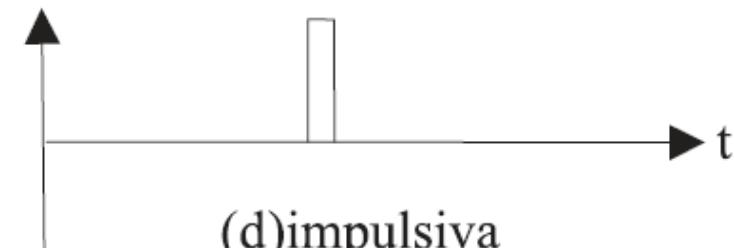
(a) harmônica



(c) transiente

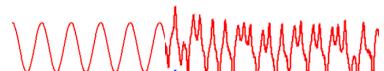


(b) periódica

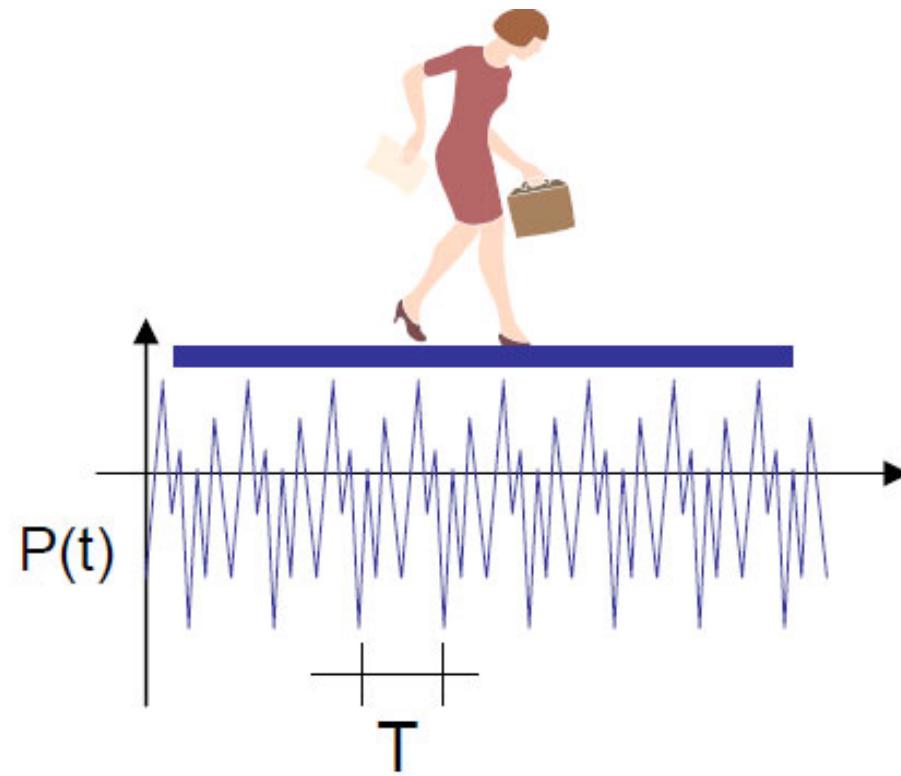


(d) impulsiva

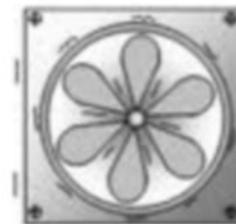
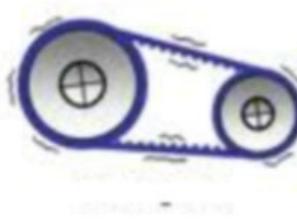
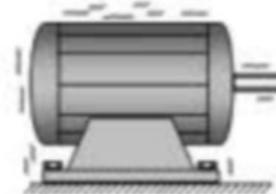
TIPOS DE CARGAS DINÂMICAS



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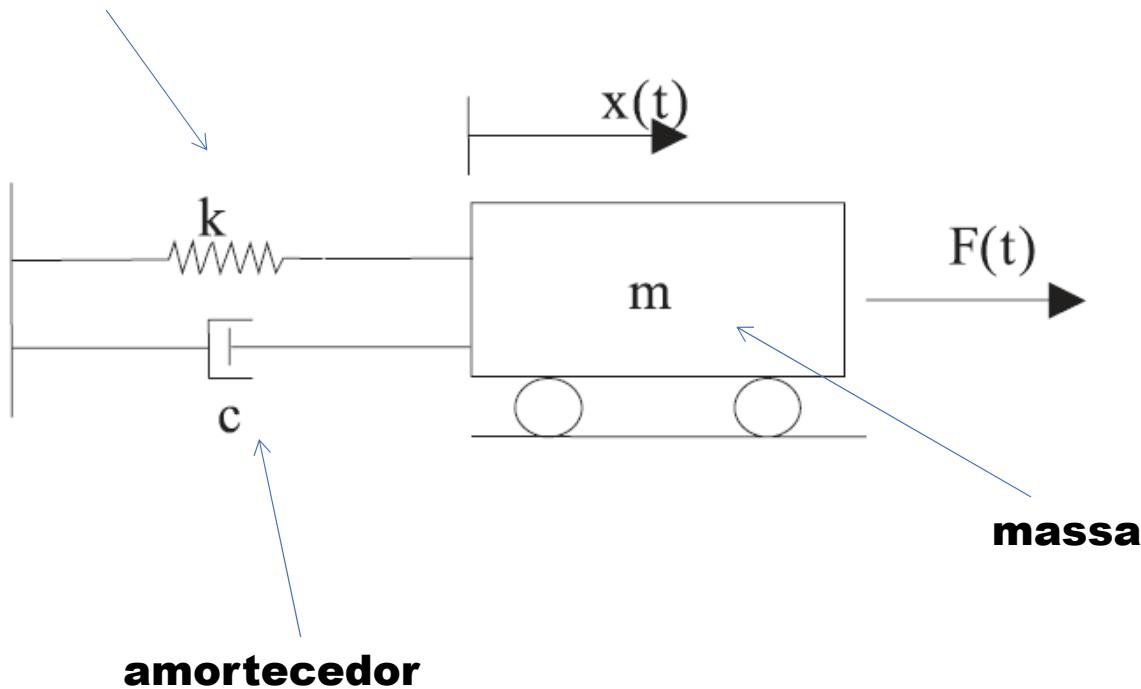


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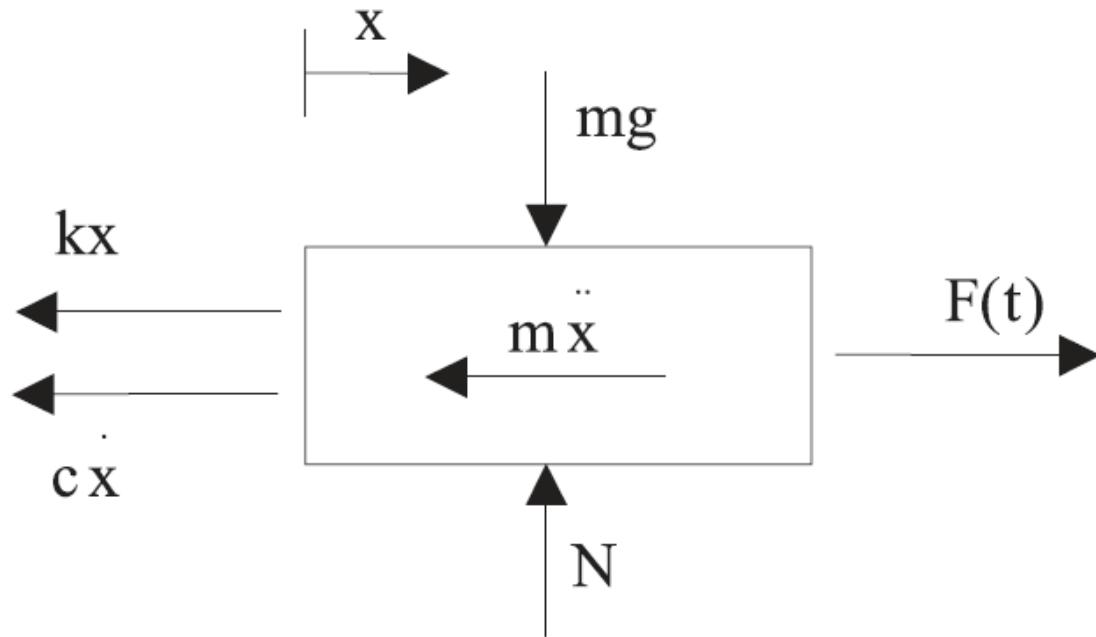


(1) SISTEMAS DE 1 GRAU DE LIBERDADE

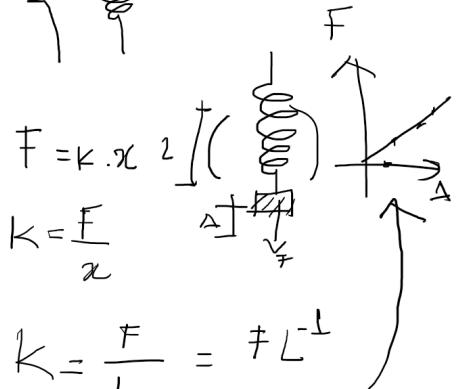
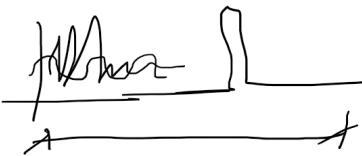
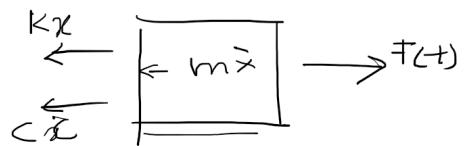
mola(rigidez)



(1) SISTEMAS DE 1 GRAU DE LIBERDADE



$$m \ddot{x}(t) + c \dot{x}(t) + kx(t) = F(t)$$



$$k = \frac{F}{L} = \frac{\text{N}}{\text{m}}$$

$$\frac{dx(t)}{dt} = v = \dot{x}$$

$$\frac{d^2x(t)}{dt^2} = a = \ddot{x}$$

$$F = k \cdot x$$

~~EQ. DYNAMIC~~

~~$m\ddot{x} + c\dot{x} + kx = F(t)$~~



$$kx = F$$

~~EQ. STATIONARY~~

$$T \rightarrow \sigma = \frac{P}{A}$$

$$\epsilon = \frac{\sigma}{E}$$

$$\sigma = E \cdot \epsilon$$

$$J = E \cdot \sigma$$

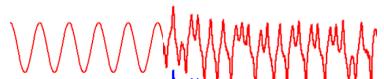
$$J = \frac{N}{mm}$$

$$S = \frac{PL}{EA}$$

$$a = \frac{F L}{E A}$$

$$F = \frac{E A}{L} x$$

Rigidizzi
Axial

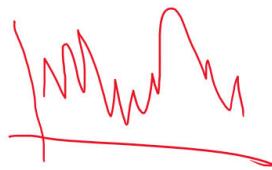


$$m\ddot{x} + c\dot{x} + kx = F(t)$$

Sens
amor

$$m\ddot{x} + kx = 0$$

$$\omega^2 = \frac{k}{m}$$



Fourier
Series

$$\ddot{x} + \frac{k}{m}x = 0$$

Eq. 2 grav

$$\rightarrow \ddot{x} + \omega^2 x = 0$$

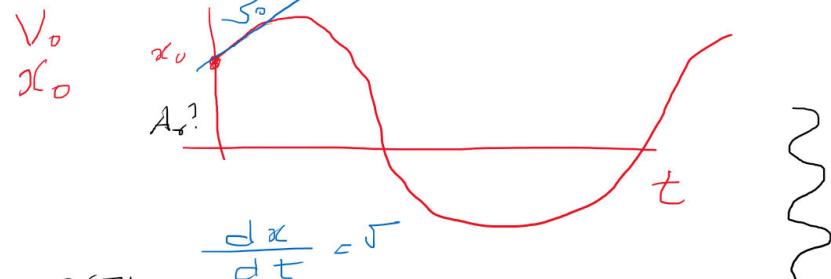
$$x = A \sin(\omega t)$$

$$-\omega^2 A \sin(\omega t) + \omega A \cos(\omega t) = 0$$

$$-\omega^2 + \omega^2 = 0 \quad \therefore \omega^2 = \frac{k}{m}$$

$$\omega = \sqrt{\frac{k}{m}}$$

Vibraciones Líquidas



$$\dot{x} = A \omega \cos(\omega t)$$

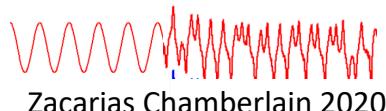
$$\ddot{x} = -A \omega^2 \sin(\omega t)$$

$$\omega = \text{freq. angular}$$

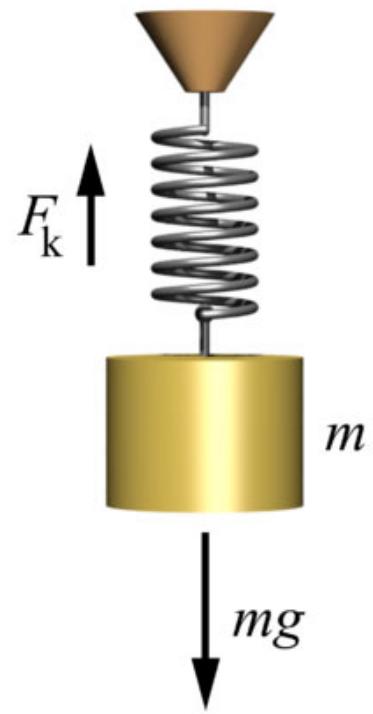
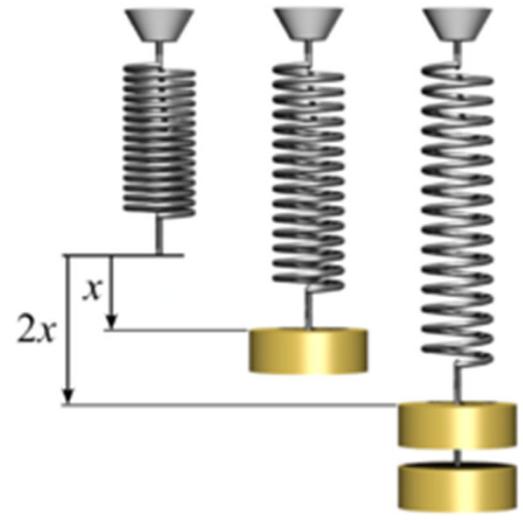
$$\omega = \sqrt{\frac{k}{m}}$$

rigido

masa



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(1) SISTEMAS DE 1 GRAU DE LIBERDADE

VIBRAÇÃO LIVRE

$$\omega_n = \sqrt{\frac{k}{m}}$$

Freqüência Circular

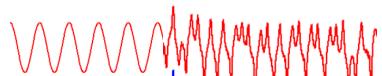
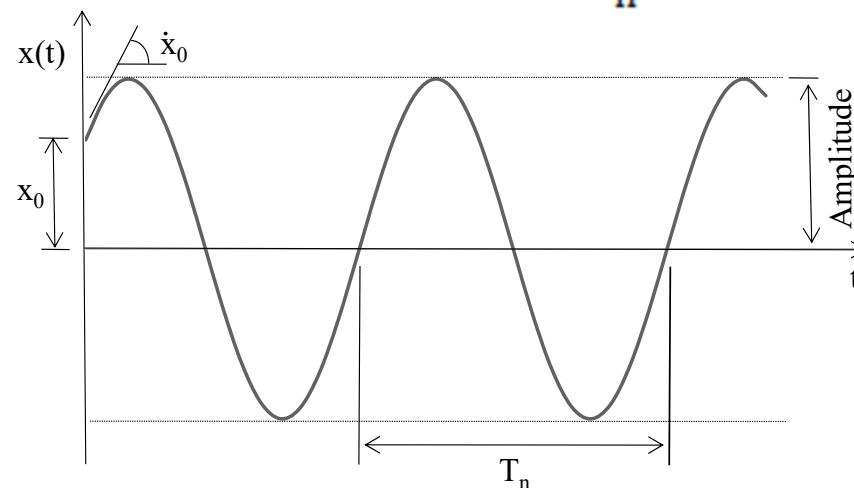
$$T_n = \frac{2\pi}{\omega_n}$$

Período Natural

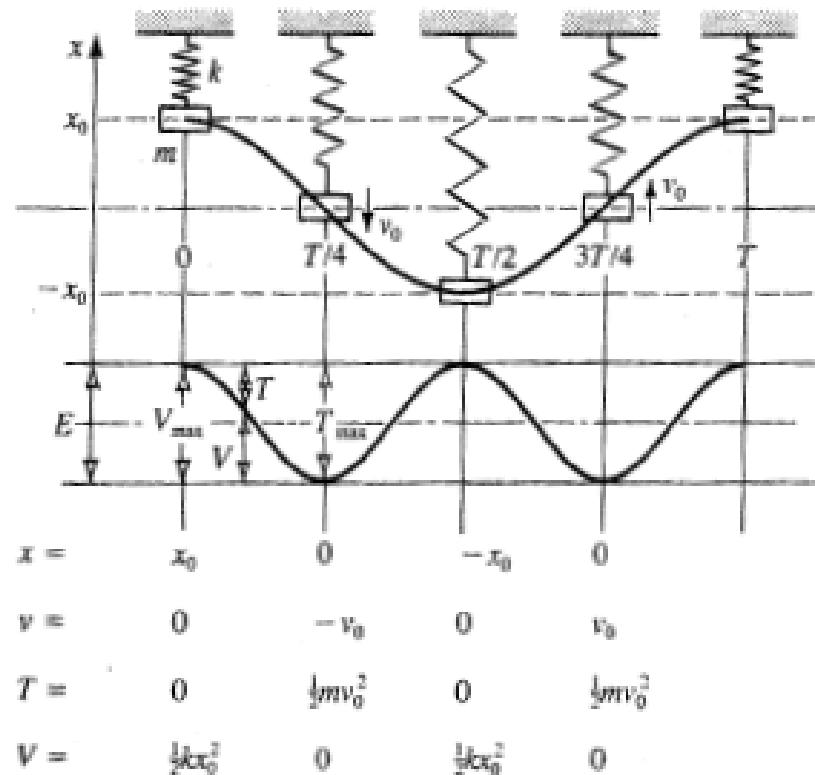
$$f_n = \frac{1}{T_n} = \frac{\omega_n}{2\pi}$$

Freqüência Natural

$$x(t) = x_0 \cos(\omega_n t) + \frac{x_0}{\omega_n} \sin(\omega_n t)$$



R #rvflodgruqflr dp ruhfldr p rwudgr qd iljxud whp #kp d dp solwgh#_3 #
 D #rvltflr p rwud xp #elfor lqwhlur frp #xdwur whp srvigh#N27#
 d ht xdtflr gh#p rylp hqwr ##gdgd#sru [#@#_3 frv+z #w,##



Exemplo 1.1: Calcular a freqüência natural da viga simplesmente apoiada de vão $I = 6,0\text{m}$, mostrada na Figura E1.1, supondo em sua seção central um equipamento com massa de $m = 760 \text{ kg}$ e que seu peso próprio seja desprezível para este problema. A viga é composta por um perfil de aço, de módulo de elasticidade $E=205\text{GPa}$, com momento de inércia $I=12258\text{cm}^4$.

Solução:

$$\frac{(7.44\text{kN})(600)^2}{(48*20500\text{kN}/\text{cm}^2*12258\text{cm}^4)} = 0,13\text{cm}$$

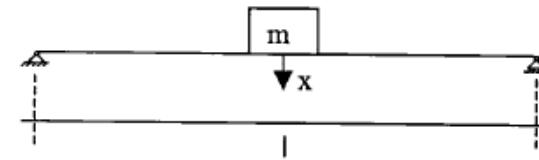
O deslocamento da seção média da viga devido a ação de uma força P nela aplicada é dada por: $\delta = \frac{Pl^3}{48EI}$.

A constante de mola equivalente é dada por:

$$k = \frac{P}{\delta} = \frac{48EI}{l^3} = 5,584 \cdot 10^6 (\text{N/m})$$

Obtendo-se para as freqüências circular e natural respectivamente:

$$\omega_n = \sqrt{\frac{k}{m}} = \sqrt{\frac{5,584 \cdot 10^6}{760}} = 85,7 \text{rad/s}; f_n = \frac{\omega_n}{2\pi} = 13,64 \text{Hz}$$



Freq. natural

$$K = \frac{F}{x}$$

$$F = mg$$

$$x = \delta$$

$$\delta = \frac{PL^3}{48EI}$$

$$K = \frac{mg}{\frac{mgL^3}{(48EI)}} = \frac{48EI}{L^3} \rightarrow K = \frac{48EI}{L^3}$$

$$E = 205000 MPa = 2.05 \times 10^{11} N/m^2$$

$$I = 12258 cm^4 \rightarrow 12258 \frac{cm^4}{100^4} m^4$$

$$m = 760 kg$$

$$(N/m)$$

$$K = \frac{48(2.05 \times 10^{11} N/m^2) \frac{12258}{100^4} m^4}{(6m)^3}$$

$$K = 5584200 N/m$$

Math

$$\omega_n = \sqrt{\frac{K}{m}}$$

$$W_{310 \times 31} \quad h$$

$$\omega_n = \sqrt{\frac{5584200 N/m}{760 kg}} = 85.7 \frac{rad}{sec}$$

$$\frac{N/m}{kg} = \cancel{kg} \cdot \frac{m/s^2}{kg} = \frac{1}{s^2}$$

$$f_n = \frac{1}{2\pi} \omega_n = \frac{85.7}{2\pi rad} \frac{rad/s}{rad} =$$

$$f_n = 13.641/s = 13.64 Hz$$

$f_n = 12.27 Hz$
 considerando massa viga